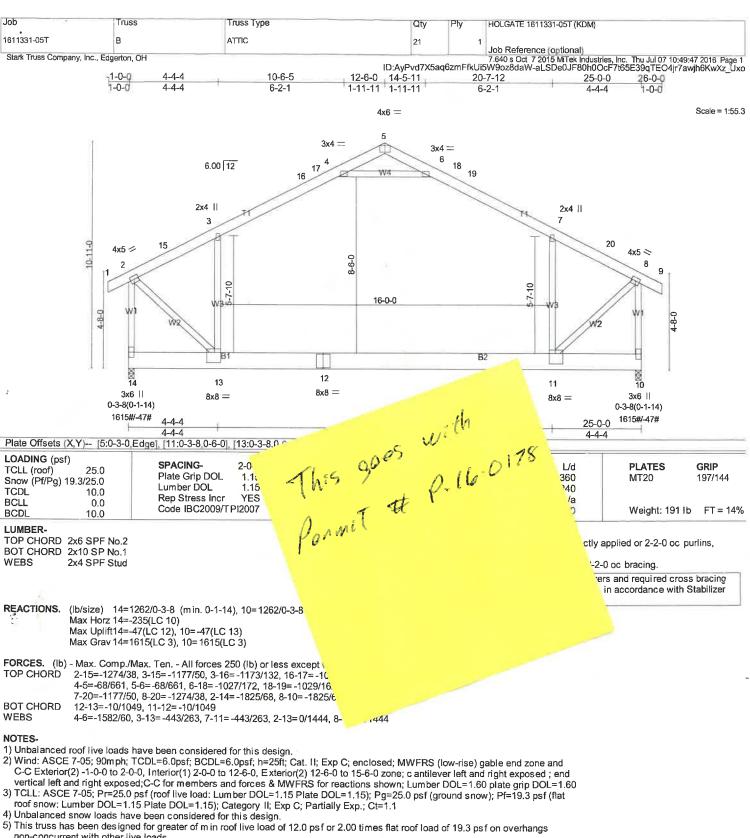
## CITY OF NAPOLEON GENERAL PERMIT APPLICATION

THIS APPLICATION IS FOR RESIDENTAL CONSTRUCTION INCLUDING BUILDING, ELECTRICAL, PLUMBING, MECHANICAL & REMODELING

DATE 5-17-16 JOB LOCATION 907 Welste	ed St
OWNER Christopher Bullock	
OWNER ADDRESS 907 Welston St No	roleon, OH 43545
CONTRACTOR & Graig Boulton	
	CELL PHONE # 419-572-19
DESCRIPTION OF WORK TO BE PERFORMED	garage
ESTIMATED COMPLETION DATE	ESTIMATED COST # 43,000,00
Affected Floor Area (AFA): In existing structures, it is the area affected by the imonly the room and not all the rooms).	pprovement, i.e. a new wall dividing a room (the AFA would be
DESCRIPTION	FEE TOTAL COST
BUILDING:	FEE TOTAL COST
Decks	\$25.00 \$
Addition & Alterations Square foot in (AFA) $x $0.05 = $$	+ \$25.00 = \$
Garage and Shed over 200 SF (Detached)	
Siding and/or Roofing	4 7.00
Windows/Doors	<b>*</b>
ELECTRICAL:	\$25.00 \$
<b>Electrical</b> Circuits in (AFA) $x $3.00/Circuit = $$	+ \$25.00 = \$
Electrical Service Upgrade	***
MECHANICAL:	\$25.00 \$
Water Heater .	£25.00 A
Furnace and/or AC Replacement	\$25.00 \$
PLUMBING:	\$25.00 \$
<b>Plumbing</b> Traps in (AFA) $x $3.00/Trap = $$	+ \$25.00 = \$
TOTAL plus Olds Book and	
TOTAL plus Ohio Board of Bu	uilding Standards Fee 1% \$ (\)
I FULLY UNDERSTAND THAT NO EXCAVATION, CONSTRUCTION OR STRUCTURAL ALTERATION OF ANY BUILDING STRUCTURE, SIGN, OR PART THEREOF AND NO USE OF THE PERMIT APPLIED FOR HEREIN LIGHT REPORTS.	TOTAL FEE: \$ 25.25
MATERIAL POR BEREIN HAS BEEN APPROVED AND ISSUED BY THE CITY OF NAPOLI	FON PLUI DINGSON DE COMBERTAKEN OR PERFORMED UNTIL THE
I hereby certify that I am the Owner of the named property, or that the proposed work is authorized by the Owne application as his her authorized agent and I agree to conform to all applicable laws of the jurisdiction. In addithe code official or the code official's authorized representative shall have the authority to enter areas covered by applicable to such permit.	er of record and that I have been all the
1 HEREBY ACKNOWLEDGE THAT I HAVE READ AND FULLY UNDERSTAND THE	ABOVE LISTED INSTRUCTIONS
SIGNATURE OF APPLICANT: Christoffer on Bulloft	DATE: 5/12/10
PRINT NAME: Christopher M. Bullock	7/1/16
BATCH# 34425 CHECK# 13418	DATE

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non-concurrent with other live loads.

6) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.

7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

8) Ceiling dead load (5.0 psf) on member(s). 3-4, 6-7, 4-6; Wall dead load (5.0psf) on member(s).3-13, 7-11 9) Bottom chord live load (30.0 psf) and additional bottom chord dead load (5.0 psf) applied only to room. 11-13

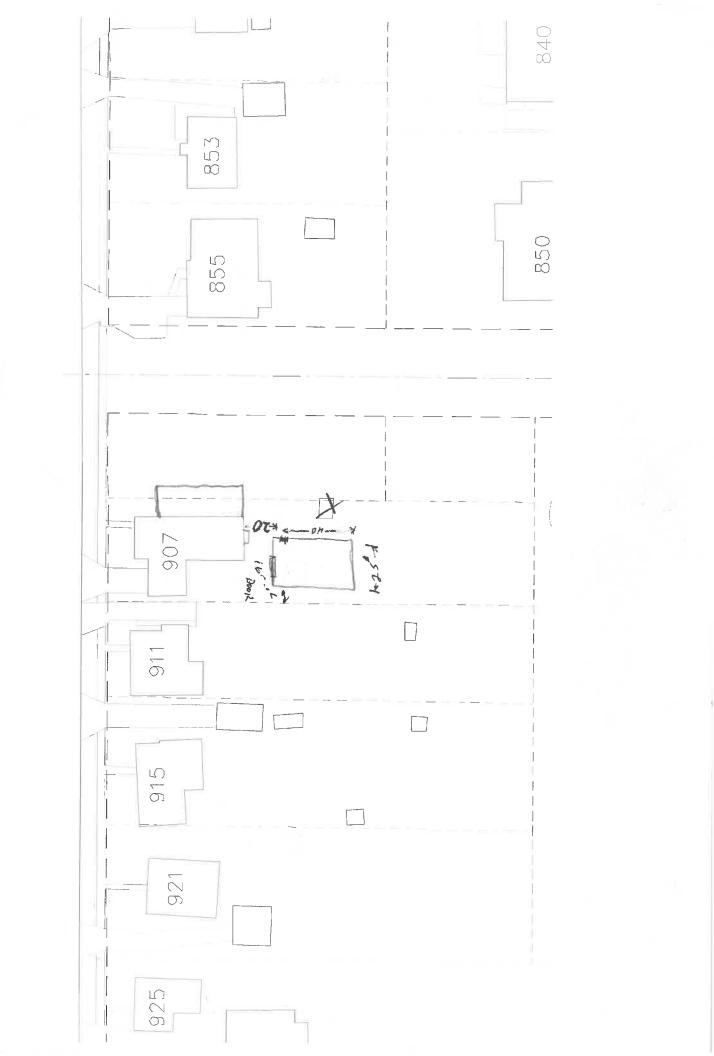
10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 47 lb uplift at joint 14 and 47 lb uplift at

11) This truss is designed in accordance with the 2009 International Building Code section 2306.1 and referenced standard ANSI/TPI

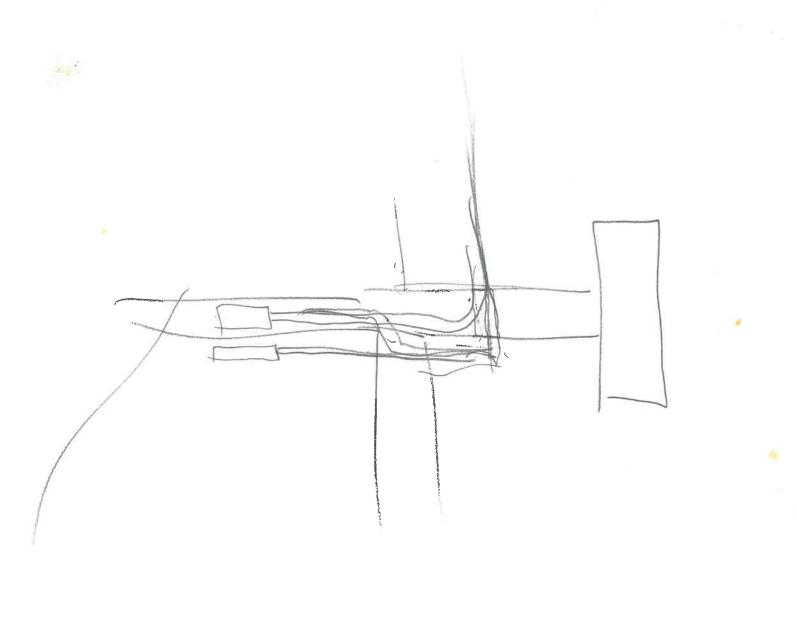
12) "Semi-rigid pitchbreaks with fixed heels" Member end fixity model was used in the analysis and design of this truss.

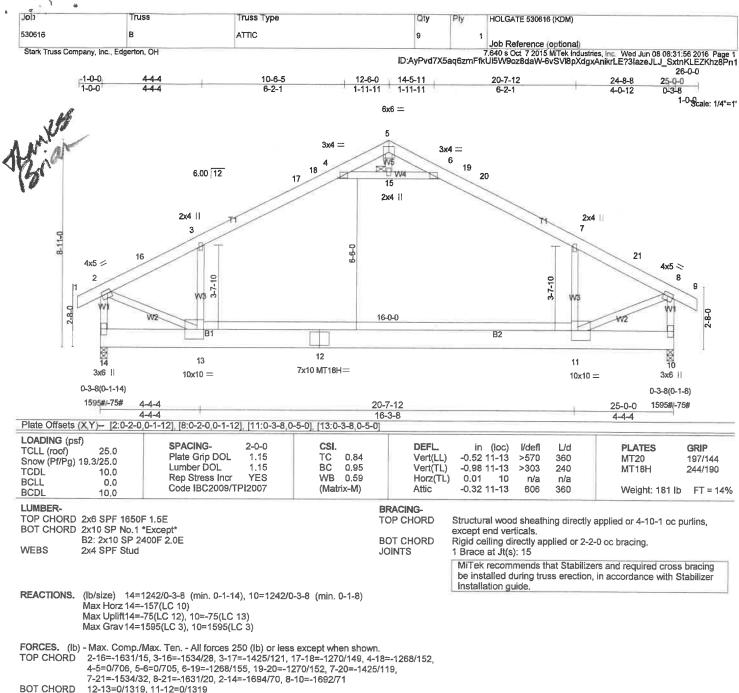
13) Attic room checked for L/360 deflection.

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(e) Y





12-13=0/1319, 11-12=0/1319

**WEBS** 4-15=-1961/49, 6-15=-1961/49, 3-13=-242/482, 7-11=-251/480, 2-13=0/1383,

8-11=0/1381

## NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
  2) Wind: ASCE 7-05; 90mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) -1-0-0 to 2-0-0, Interior(1) 2-0-0 to 12-6-0, Exterior(2) 12-6-0 to 15-6-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 at Table 20 plate grip DOL=1.50 plate grip DOL=1.50 plate grip DOL=1.15 Plate DOL=1.15 Plate DOL=1.15; Pg=25.0 psf (ground snow); Pf=19.3 psf (flat roof snow; Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp C; Partially Exp.; Ct=1.1
  4) Unbalanced snow loads have been considered for this design.

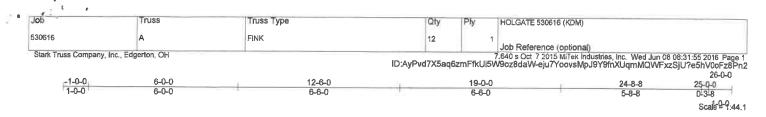
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 19.3 psf on overhangs non-concurrent with other live loads.
- 6) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.

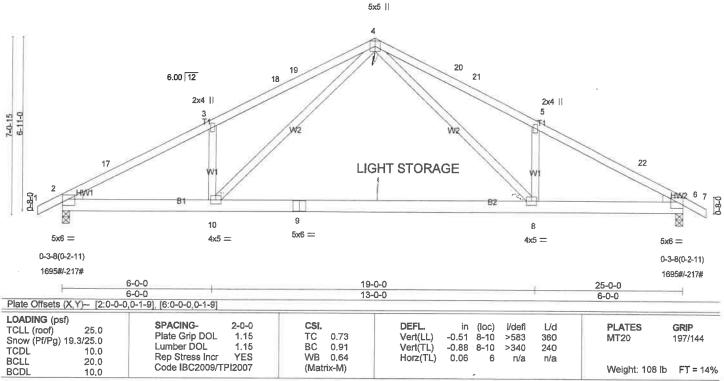
7) All plates are MT20 plates unless otherwise indicated.

- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) Ceiling dead load (5.0 psf) on member(s). 3-4, 6-7, 4-15, 6-15; Wall dead load (5.0 psf) on member(s).3-13, 7-11 10) Bottom chord live load (30.0 psf) and additional bottom chord dead load (5.0 psf) applied only to room, 11-13
- 11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 14, 10.

  12) This truss is designed in accordance with the 2009 International Building Code section 2306.1 and referenced standard ANSI/TPI
- 13) "Semi-rigid pitchbreaks with fixed heels" Member end fixity model was used in the analysis and design of this truss.
- 14) Attic room checked for L/360 deflection.

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**BRACING-**

TOP CHORD

**BOT CHORD** 

Structural wood sheathing directly applied or 3-0-7 oc purlins.

MiTek recommends that Stabilizers and required cross bracing

be installed during truss erection, in accordance with Stabilizer

Rigid ceiling directly applied or 10-0-0 oc bracing.

Installation guide.

LUMBER-

TOP CHORD 2x4 SPF 1650F 1.5E BOT CHORD 2x4 SFF 1650F 1.5E BOT CHORD 2x6 SPF No.2 \*Except\* B2: 2x6 SPF 1650F 1.5E

WEBS

2x4 SPF Stud

WEDGE

Left: 2x4 SPF Stud, Right: 2x4 SPF Stud

(lb/size) 2=1695/0-3-8 (min. 0-2-11), 6=1695/0-3-8 (min. 0-2-11) REACTIONS.

Max Horz 2=-97(LC 11)

Max Uplift2=-217(LC 10), 6=-217(LC 11)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-17=-2875/234, 3-17=-2712/255, 3-18=-2878/392, 18-19=-2776/396, 4-19=-2768/412.

4-20=-2776/411, 20-21=-2783/395, 5-21=-2885/391, 5-22=-2719/254, 6-22=-2882/233

**BOT CHORD** 2-10=-223/2483, 9-10=-83/1473, 8-9=-83/1473, 6-8=-125/2489 WEBS 4-8=-196/1478, 5-8=-431/272, 4-10=-198/1468, 3-10=-430/272

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-05; 90mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) -1-0-0 to 2-0-0, Interior(1) 2-0-0 to 12-6-0, Exterior(2) 12-6-0 to 15-6-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-05; Pr=25.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=25.0 psf (ground snow); Pf=19.3 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp C; Partially Exp.; Ct=1.1

4) Unbalanced snow loads have been considered for this design.

5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 19.3 psf on overhangs non-concurrent with other live loads.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

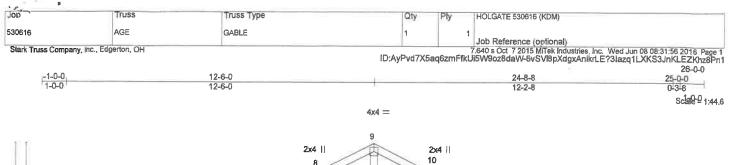
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except ([t=lb] 2=217.6=217.

8) This truss is designed in accordance with the 2009 International Building Code section 2306.1 and referenced standard ANSI/TPI 1.

9) "Semi-rigid pitchbreaks with fixed heels" Member end fixity model was used in the analysis and design of this truss.

LOAD CASE(S) Standard

		Σ.
		91



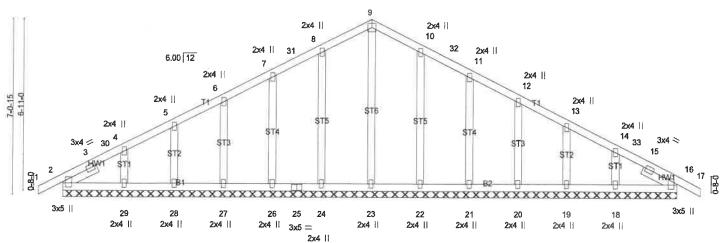


Plate Offsets (X,Y) [2:0-2-0	0.4.51 [46:0.2.0.4.5]	25-0	-o					
LOADING (psf) TCLL (roof) 25.0 Snow (Pf/Pg) 19.3/25.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr. YES Code IBC2009/TPI2007	CSI. TC 0.09 BC 0.03 WB 0.12 (Matrix)	Vert(TL) -0	in (loc) .00 16 .00 16 .00 16	I/defl n/r n/r n/a	L/d 180 90 n/a	PLATES MT20 Weight: 112 lb	<b>GRIP</b> 197/144 FT = 14%

25-0-0

LUMBER-

TOP CHORD 2x4 SPF No.2 BOT CHORD 2x4 SPF No.2 2x4 SPF Stud **OTHERS** 

SLIDER Left 2x4 SPF Stud 1-6-7, Right 2x4 SPF Stud 1-6-7 **BRACING-**

TOP CHORD **BOT CHORD**  Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS. All bearings 25-0-0.

(lb) - Max Horz 2=-89(LC 11)

Max Uplift All uplift 100 lb or less at joint(s) 2, 24, 26, 27, 28, 29, 22, 21, 20, 19, 18, 16 Max Grav All reactions 250 lb or less at joint(s) 2, 23, 24, 26, 27, 28, 29, 22, 21, 20, 19, 18, 16

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

## NOTES-

- NOTES1) Unbalanced roof live loads have been considered for this design.
  2) Wind: ASCE 7-05; 90mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) -1-0-0 to 2-0-0, Interior(1) 2-0-0 to 12-6-0, Exterior(2) 12-6-0 to 15-6-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
  3) Truss designed for wind loads in the plane of the truss only. For stude exposed to wind (normal to the face), see Standard Industry Cable For Policible or consult qualified building designer as per ANSI/TPI 1.
- Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

  4) TCLL: ASCE 7-05; Pr=25.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=25.0 psf (ground snow); Pf=19.3 psf (flat
- roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp C; Partially Exp.; Ct=1.1 5) Unbalanced snow loads have been considered for this design.

6) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 19.3 psf on overhangs non-concurrent with other live loads.

7) Gable requires continuous bottom chord bearing.

8) Gable studs spaced at 2-0-0 oc.

9) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 24, 26, 27, 28,
- 11) This truss is designed in accordance with the 2009 International Building Code section 2306.1 and referenced standard ANSI/TPI
- 12) "Semi-rigid pitchbreaks with fixed heels" Member end fixity model was used in the analysis and design of this truss.

LOAD CASE(S) Standard

			JF =